

Western Bridge Engineers' Seminar

9/27/2011

Phoenix, AZ

FPR Deck Structural Evaluation for the Morrison Bridge

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Multnomah County

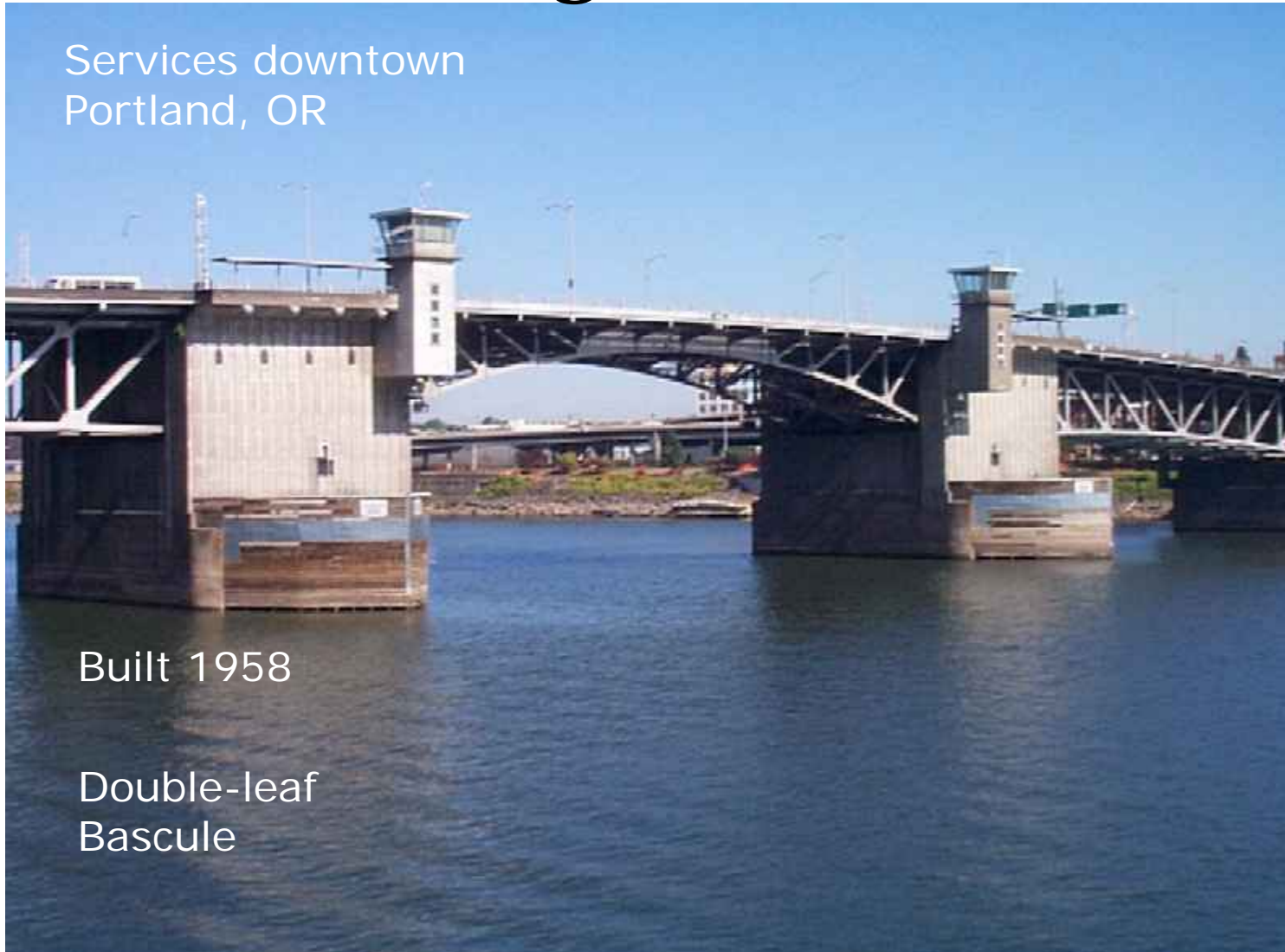
Peter Dusicka
Portland State University

Holly Winston
Oregon Department
of Transportation



Morrison Bridge

Services downtown
Portland, OR



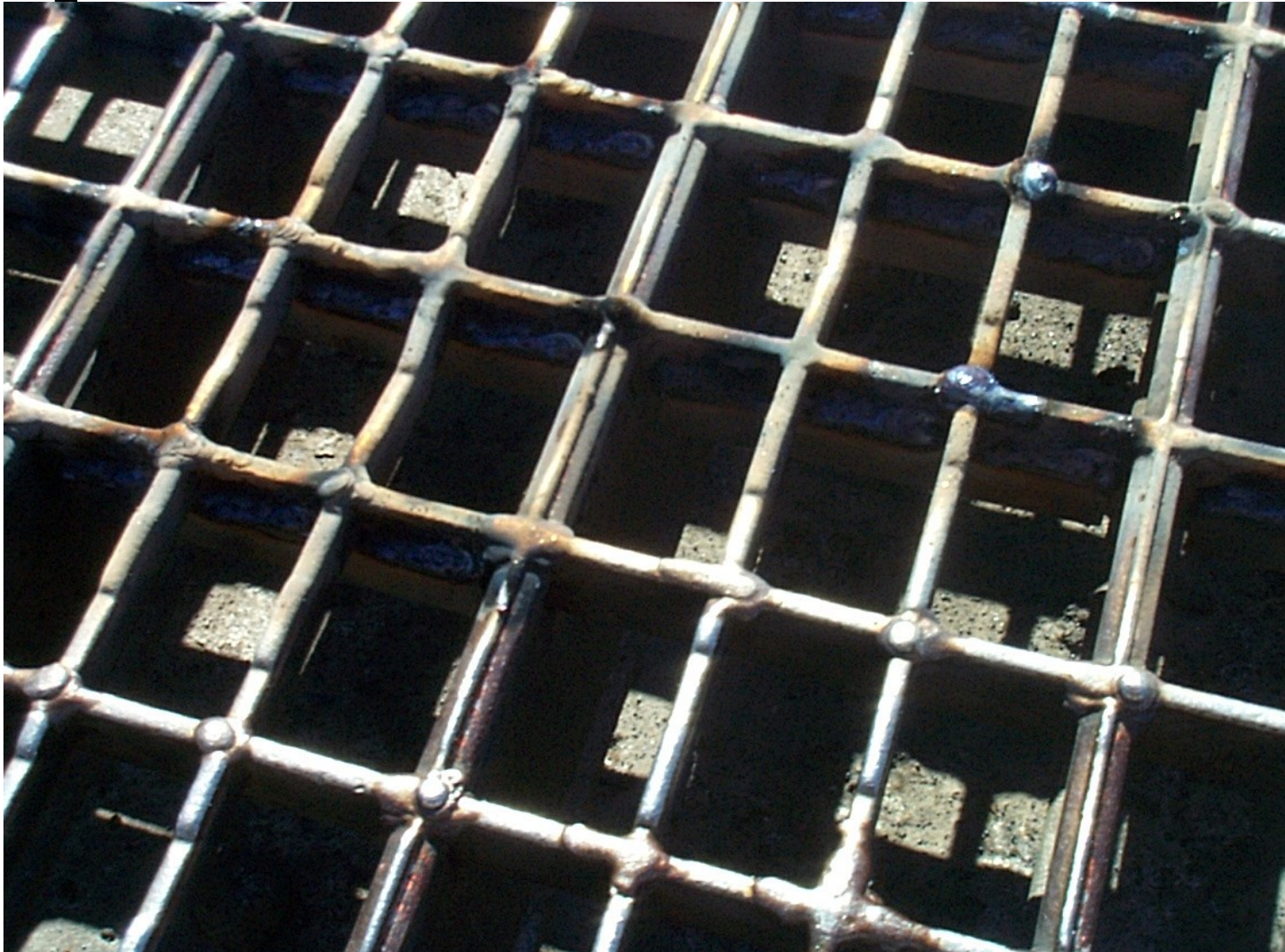
Built 1958

Double-leaf
Bascule

Existing Bridge Deck



Replacement Need - Maintenance



Replacement Need - Safety



Replacement Need - Environmental



Composite Decks



Structural Evaluation Objectives

FRP deck panels:

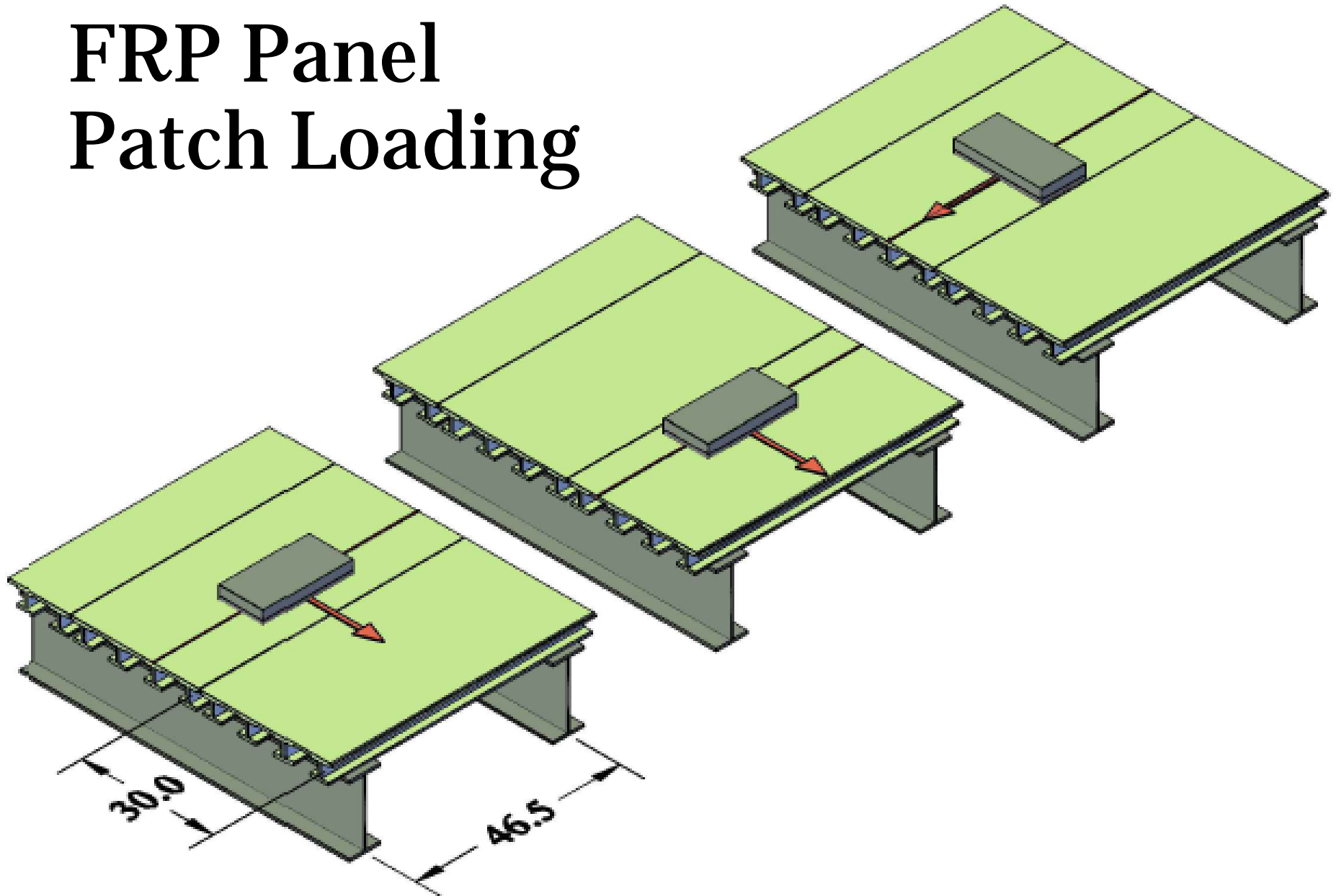
- strength and stiffness
- failure modes
- fatigue characteristics



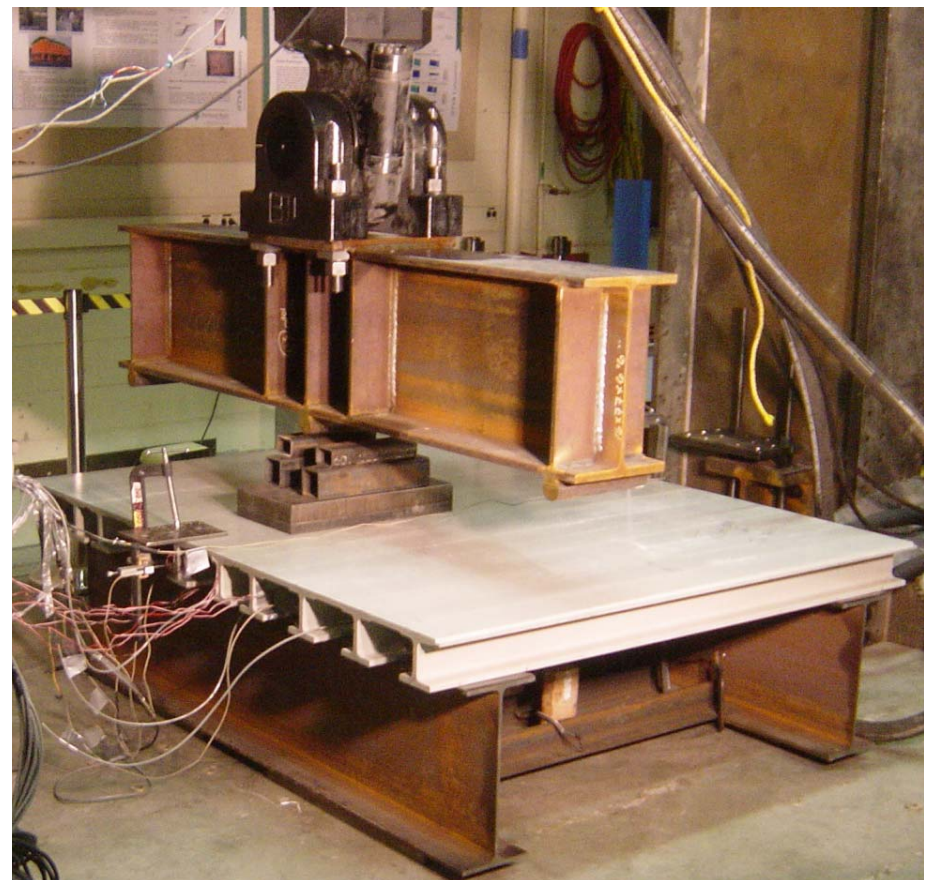
Panel to stringer bolted connection:

- strength
- failure modes

FRP Panel Patch Loading

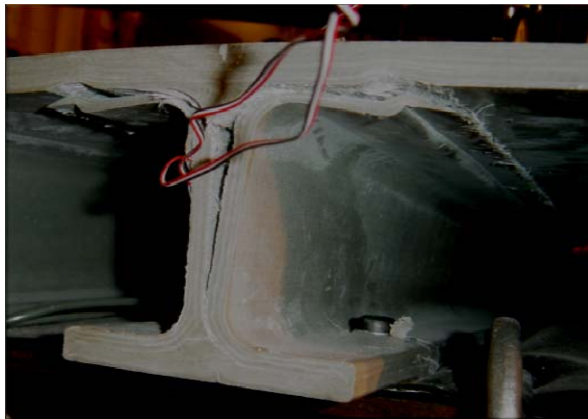


FRP Panel Strength Evaluation



Flexural Failure Modes

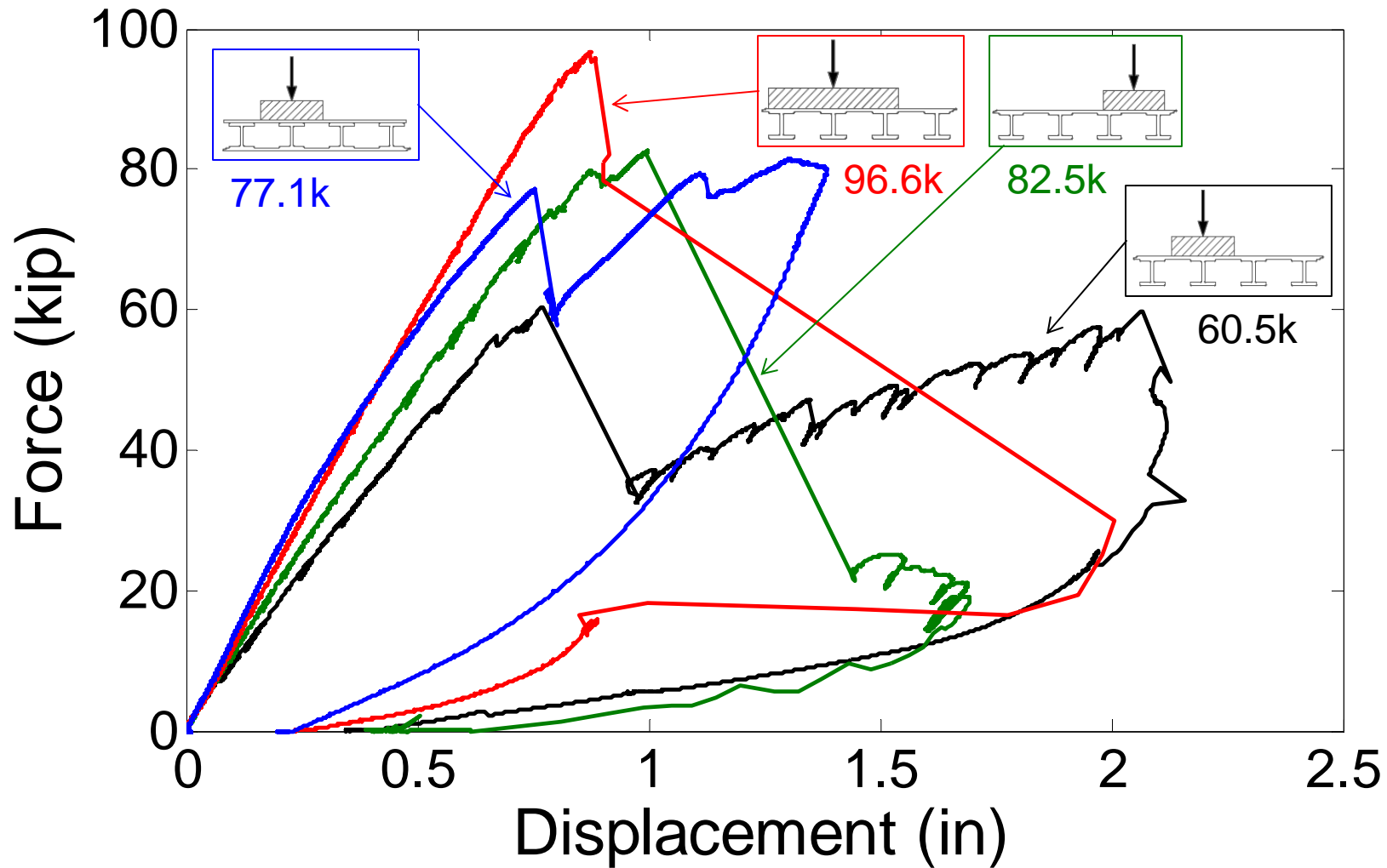
1. Delamination



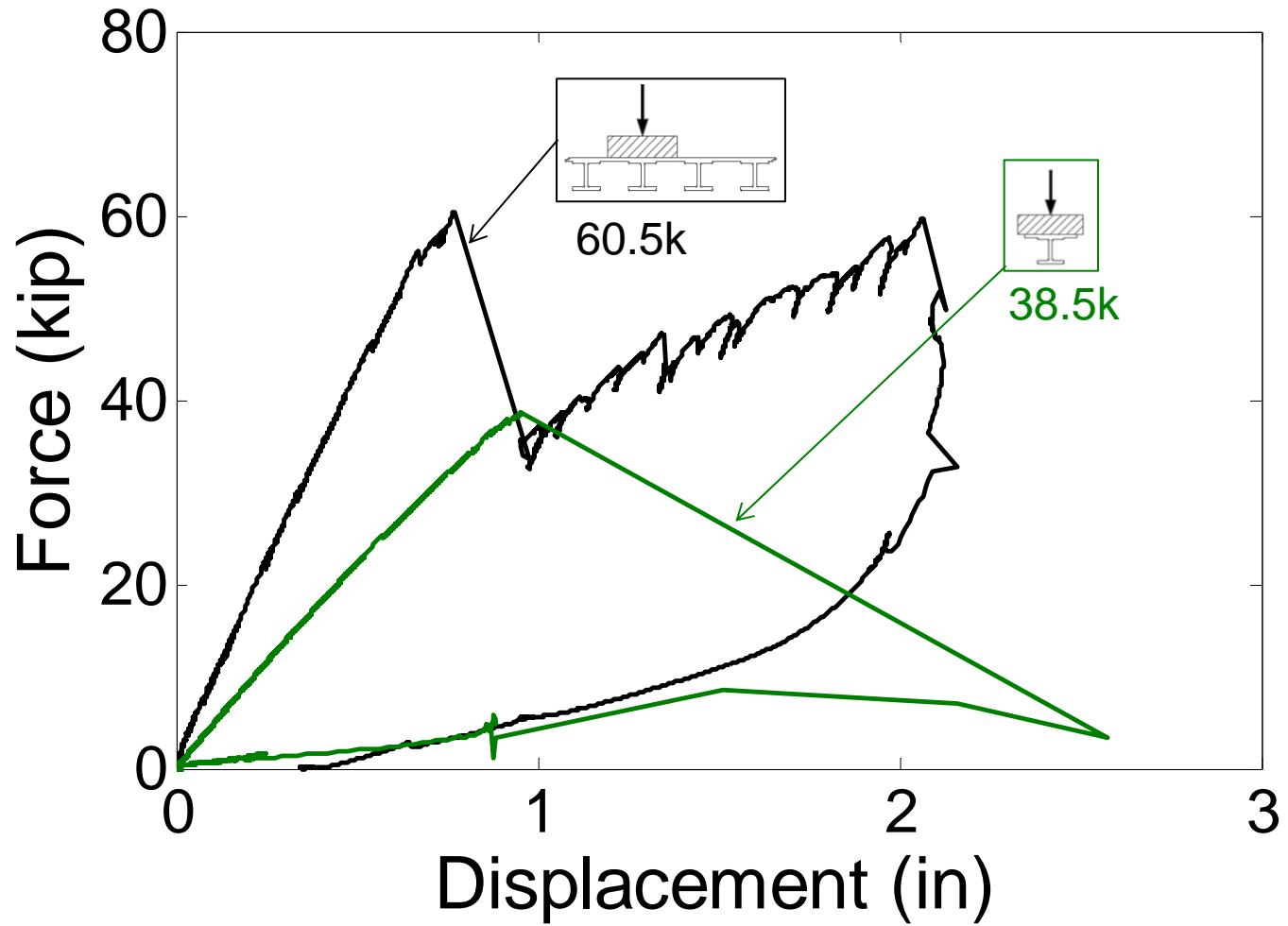
2. Web Crushing



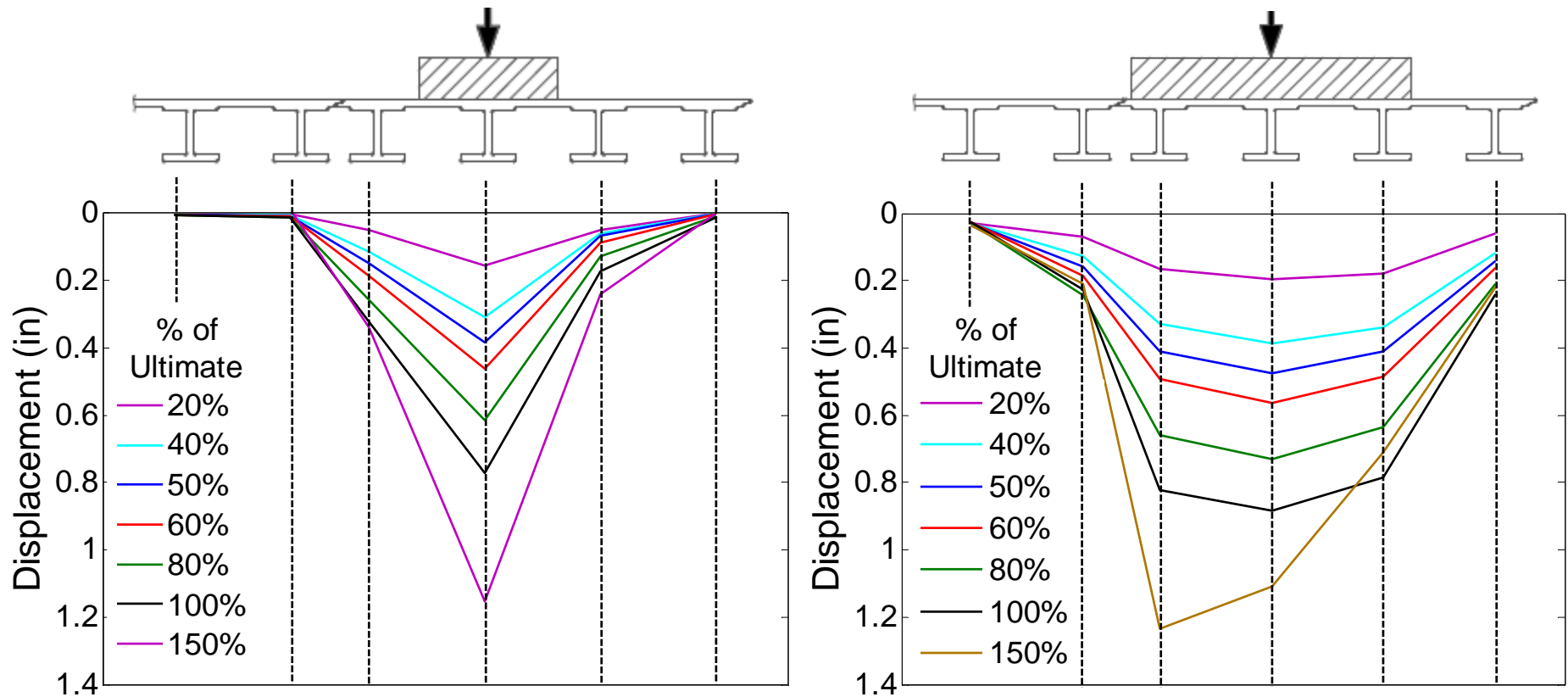
Flexural Behavior



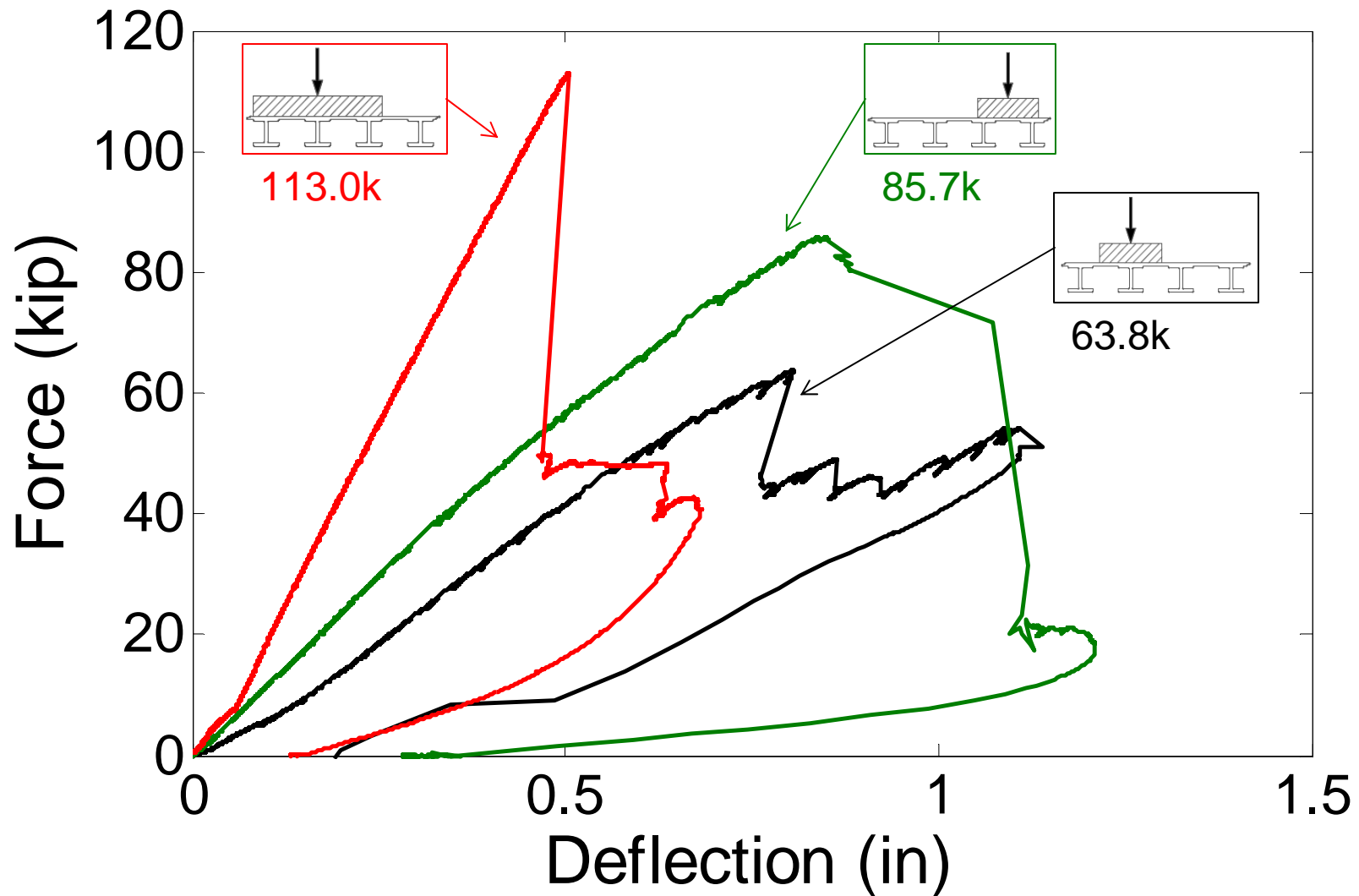
Beam to Panel Sharing



Flexural Distribution in Beams



Shear Behavior



Shear – Failure Modes

1. Delamination

2. Web Crushing



Fatigue Evaluation Approach

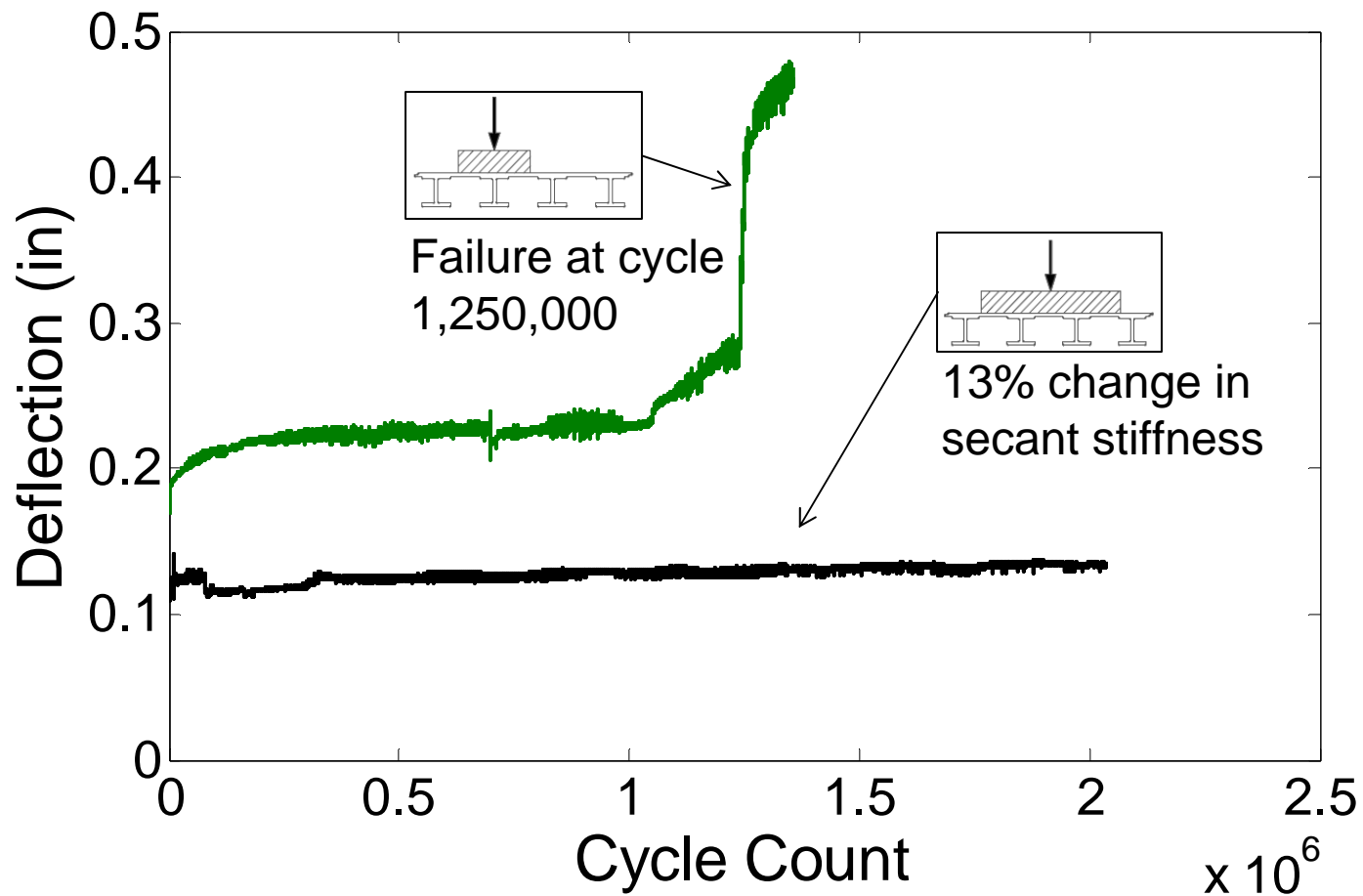
- HITEC approach
 - Load @ 1.5 times the wheel load (24kip)
 - Cycles @ 2 million cycles

- FHWA approach
 - Load @ AASHTO fatigue limit state (13.8kip)
 - Cycles @ Based on traffic demand (6.16 million cycles)

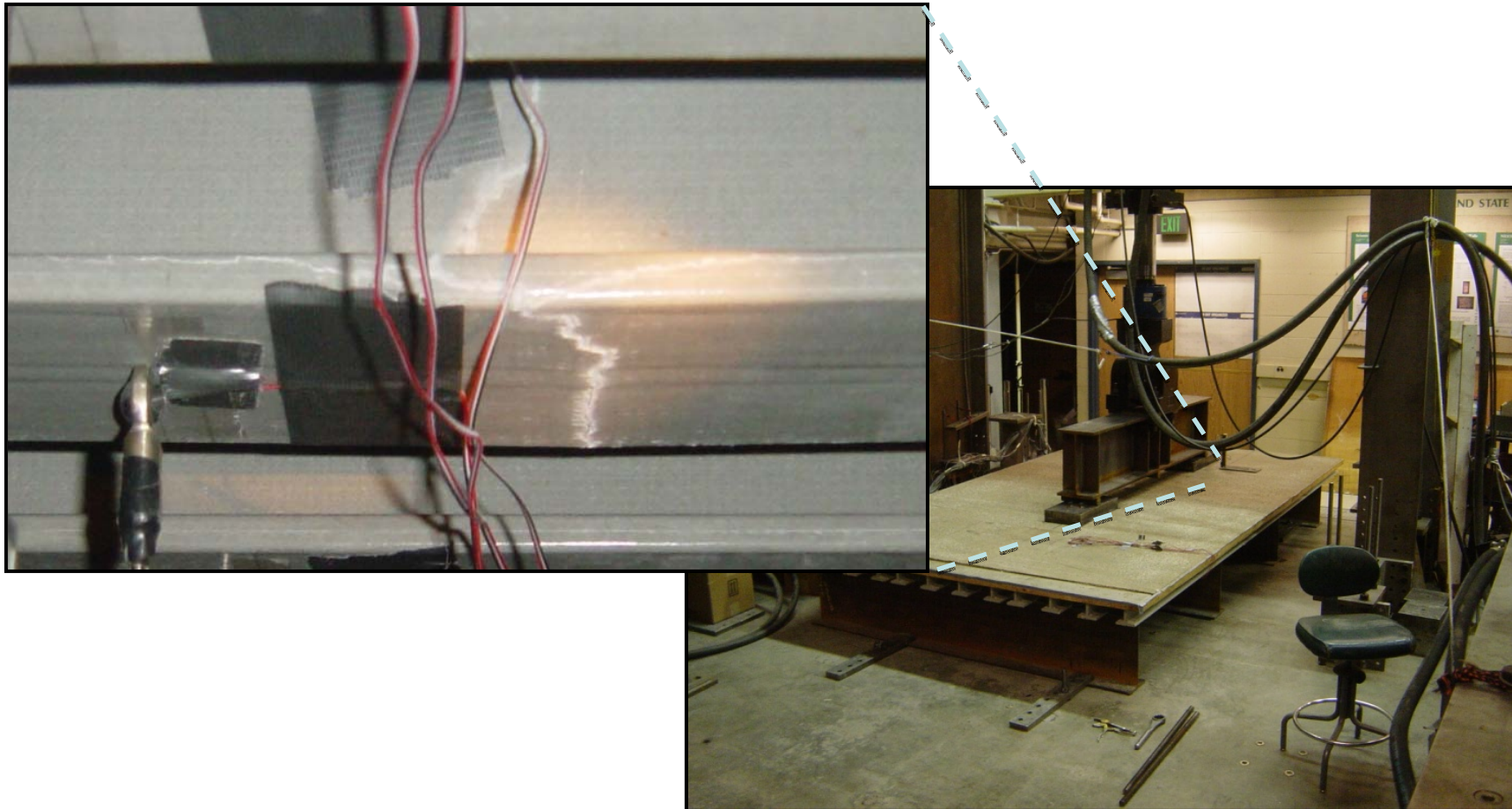
Fatigue – Axle Load Test Setup



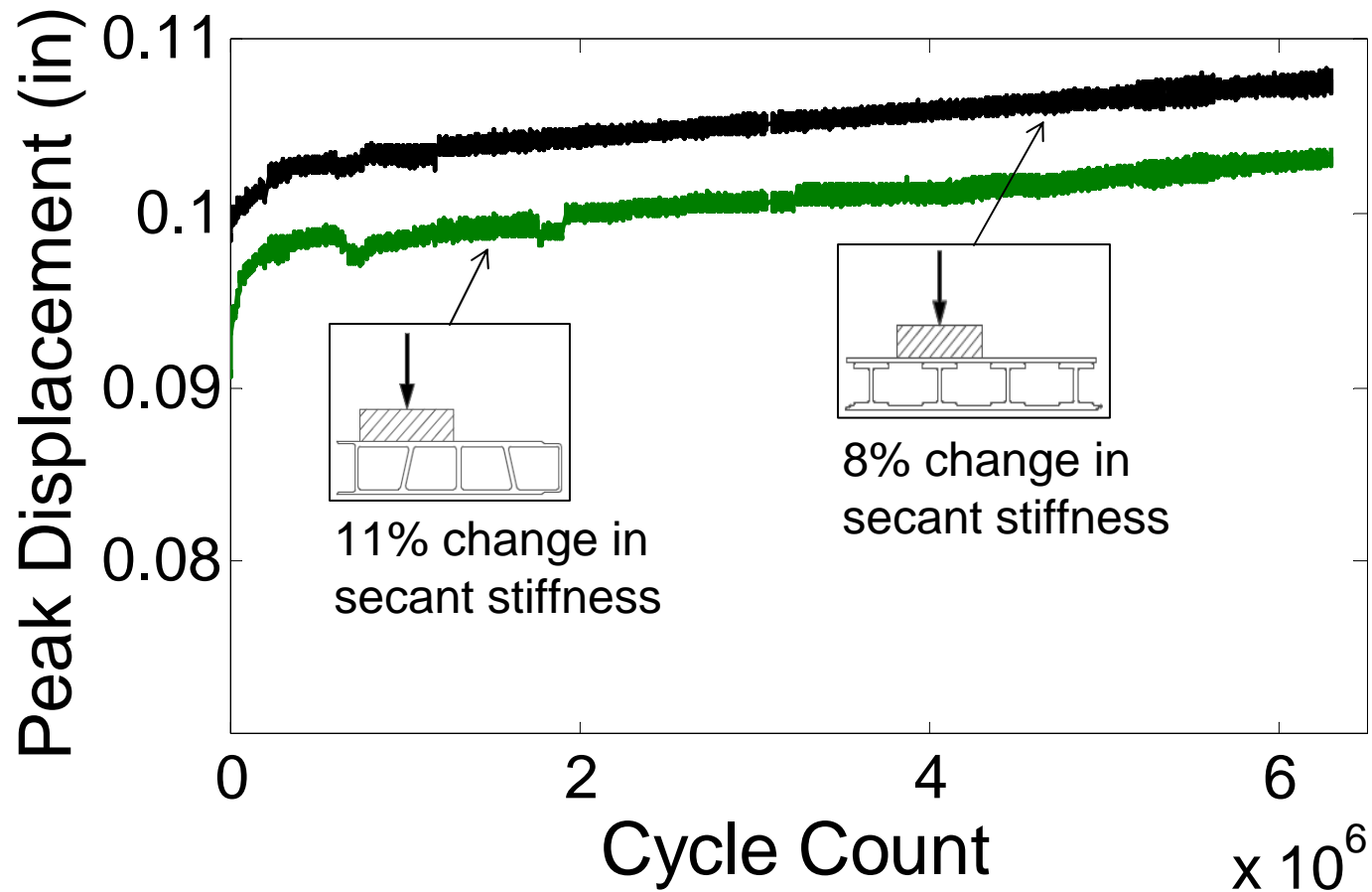
HITEC – Inverted T-beam Deck



Fatigue Failure Mode



FHWA – Closed Box Decks



Bolted Connections

Pull

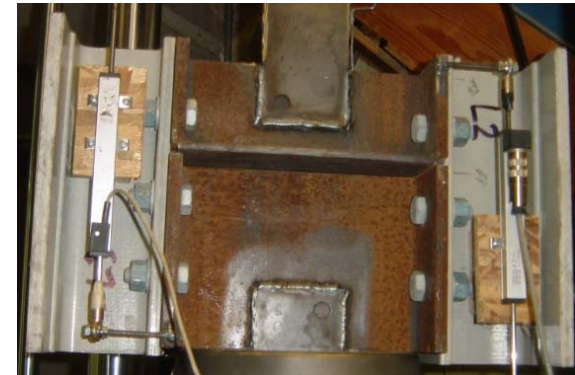
Shear
Perpendicular
to Fibers

Shear
Parallel
to Fibers

Martin Marietta



Zellcomp



Pull Test

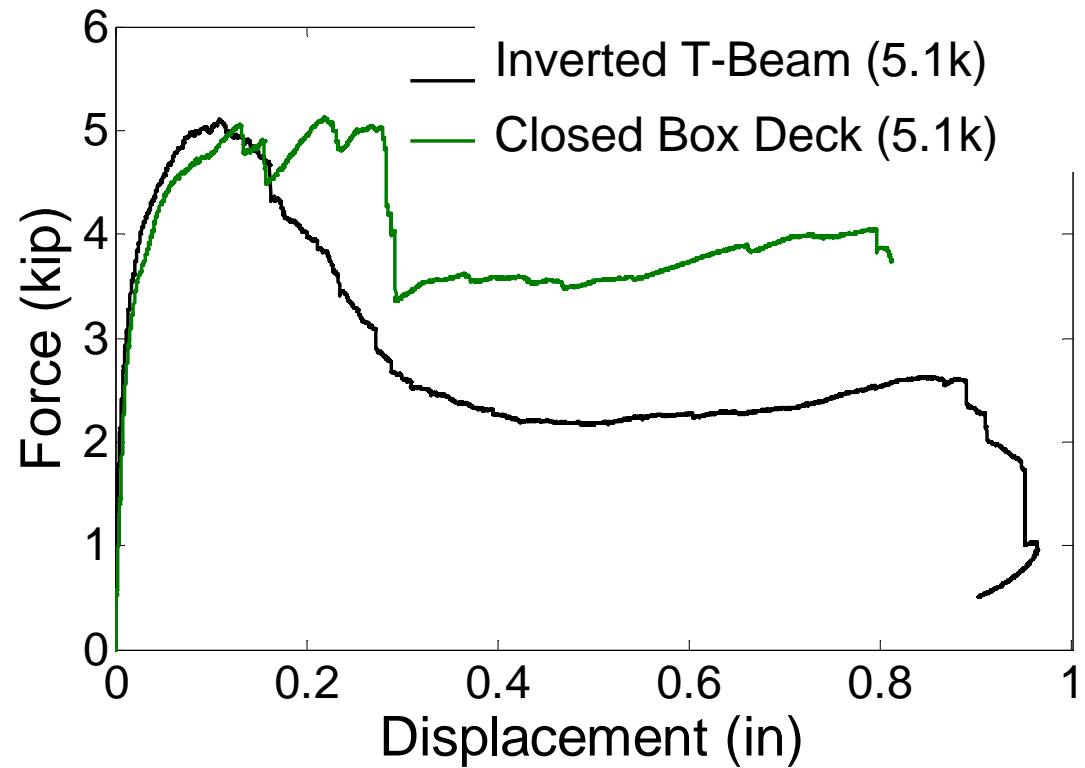
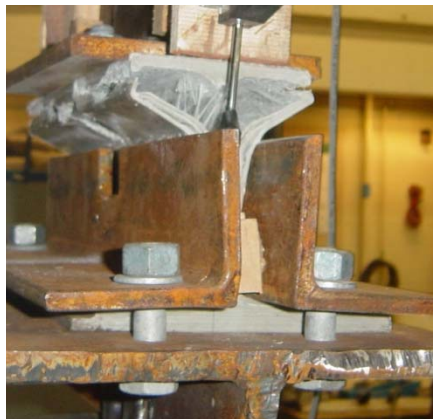
Closed Box (section)

Failure Mode:
Delamination of web



Inverted T-beam

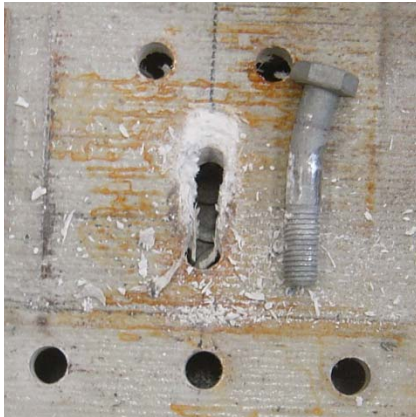
Failure Mode:
Delamination of web



Shear Perpendicular to Fibers

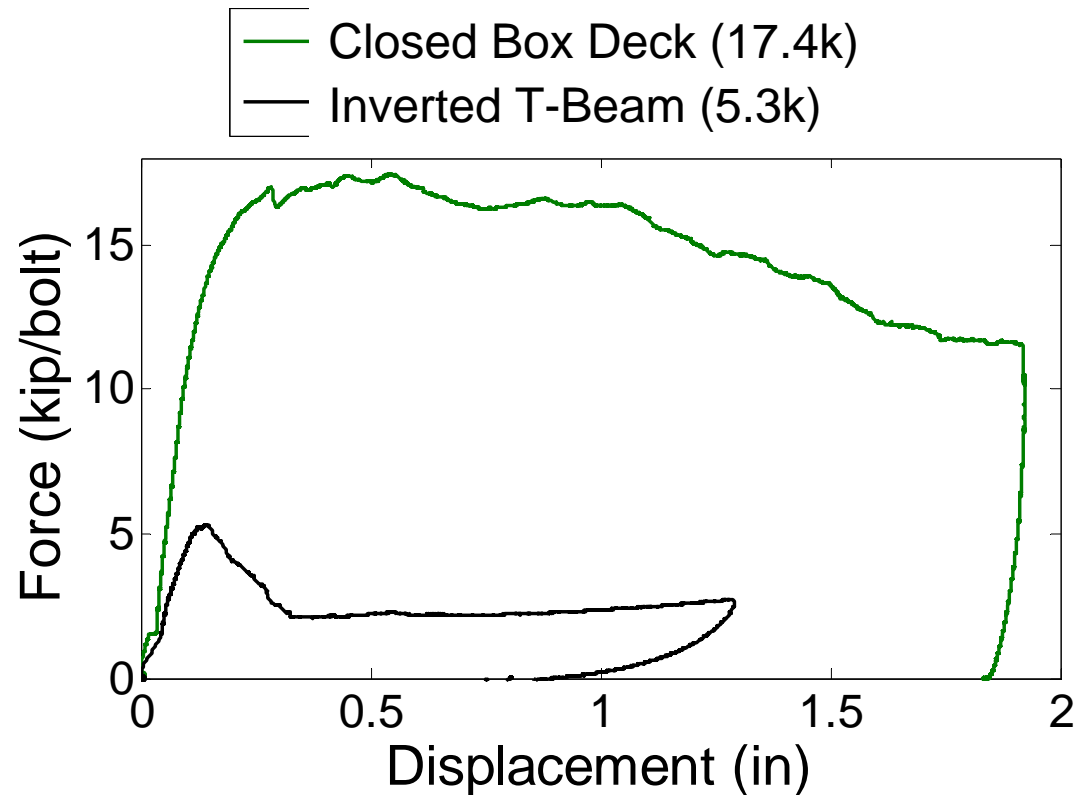
Closed Box (section)

Failure Mode:
Bolt Bearing



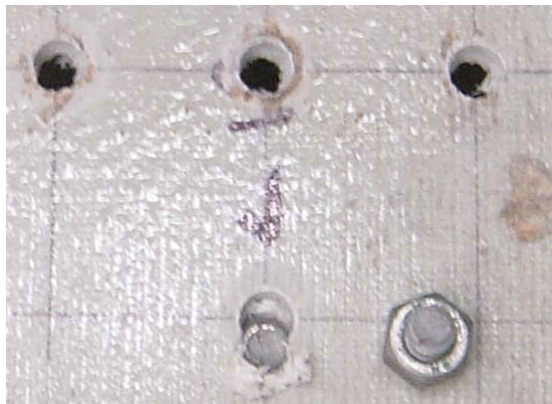
Inverted T-beam

Failure Mode:
Bolt Bearing

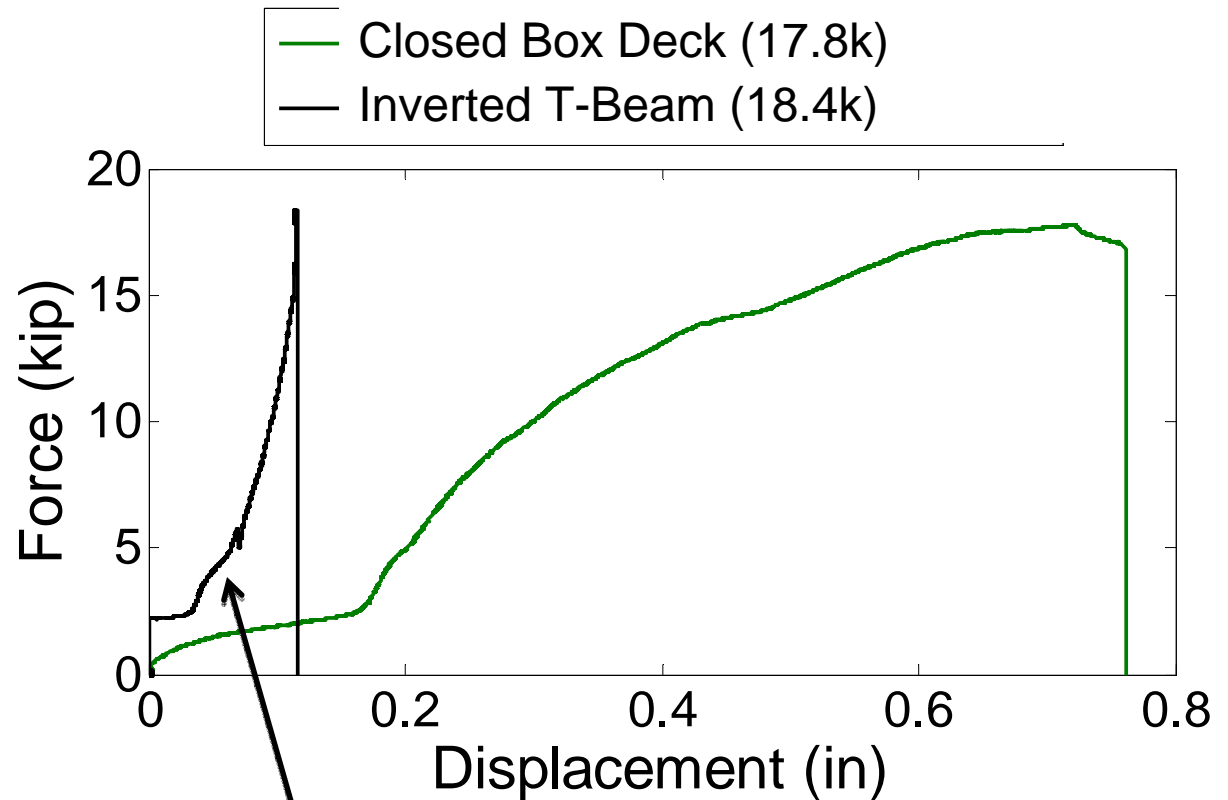


Shear Parallel to Fibers

Closed Box (section)
Failure Mode: Bolt shear
(threads not excluded)



Inverted T-beam
Failure Mode:
Flange Delamination



Note: bolt closer to and engaging web, resulting in smaller displacements.

Deck to Girder Strength - Specimens

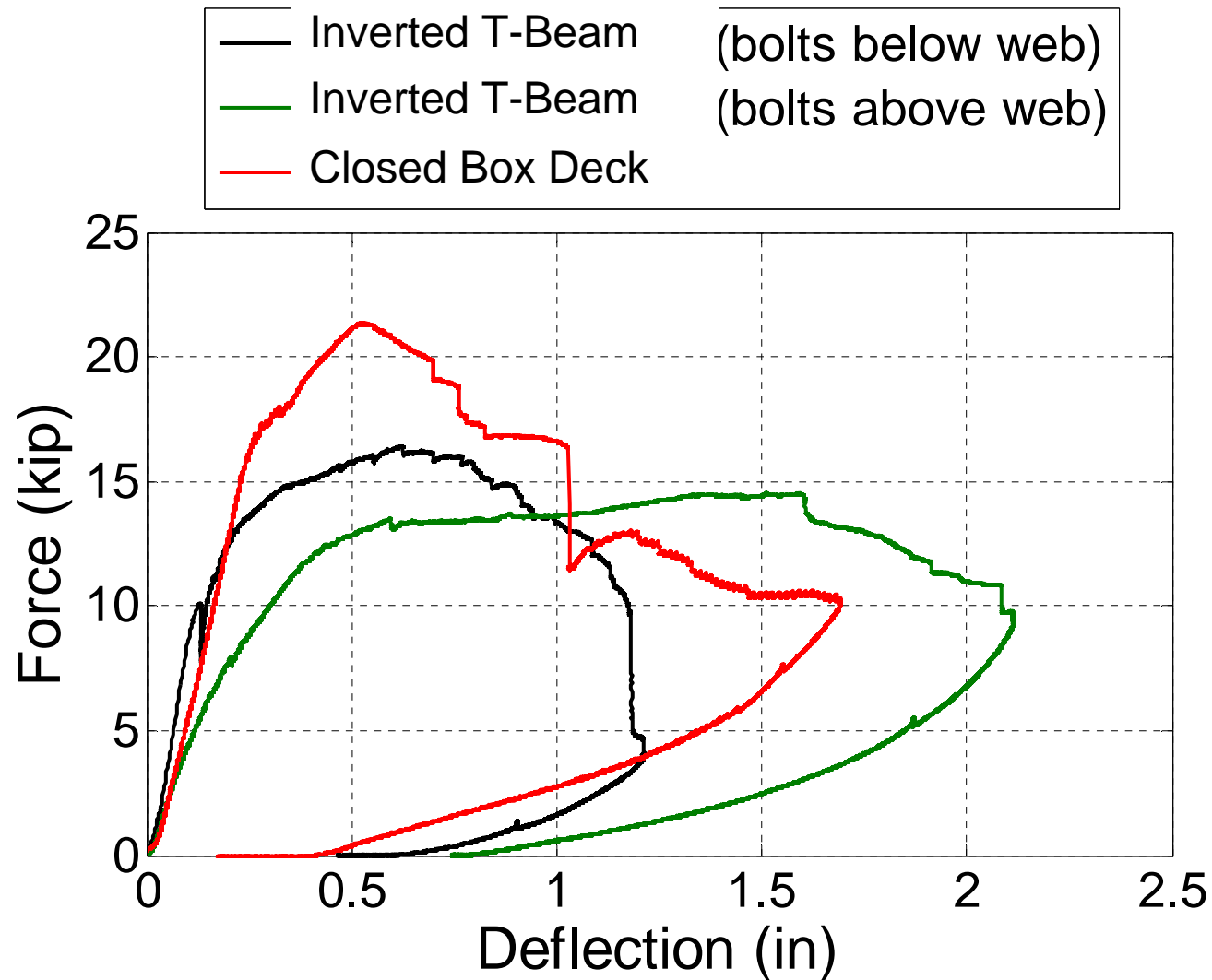
Inverted T-Beam
Deck



Closed Box
Deck



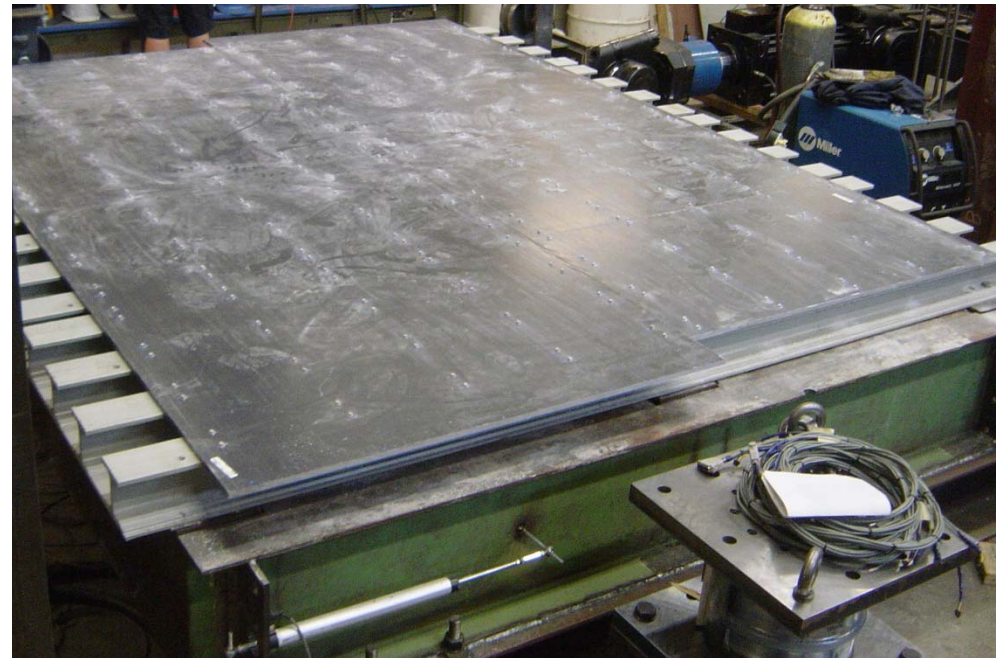
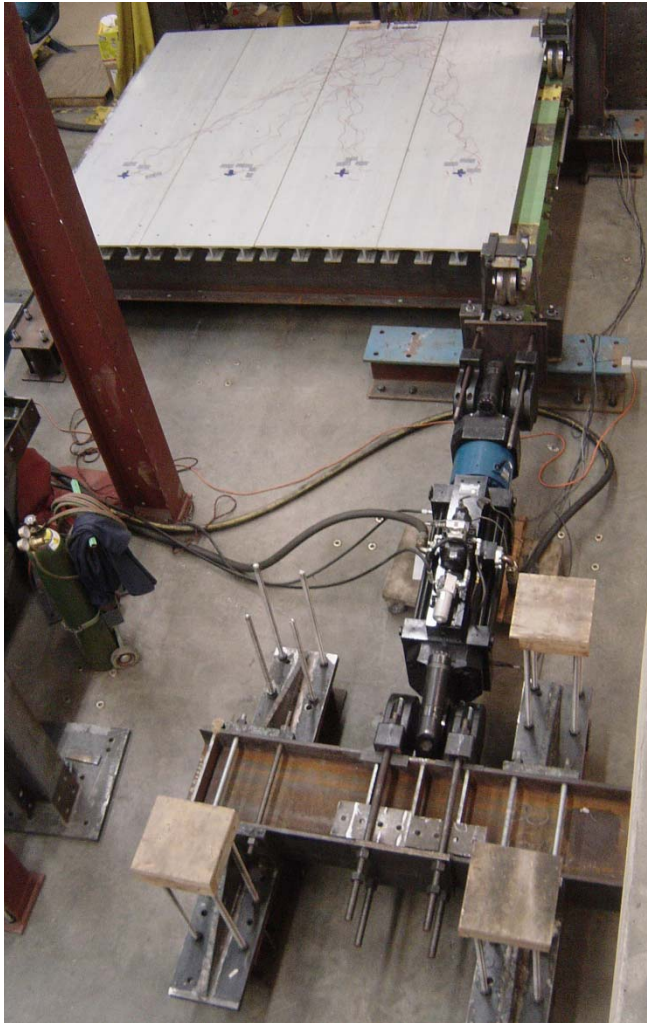
Deck to Girder Test Results



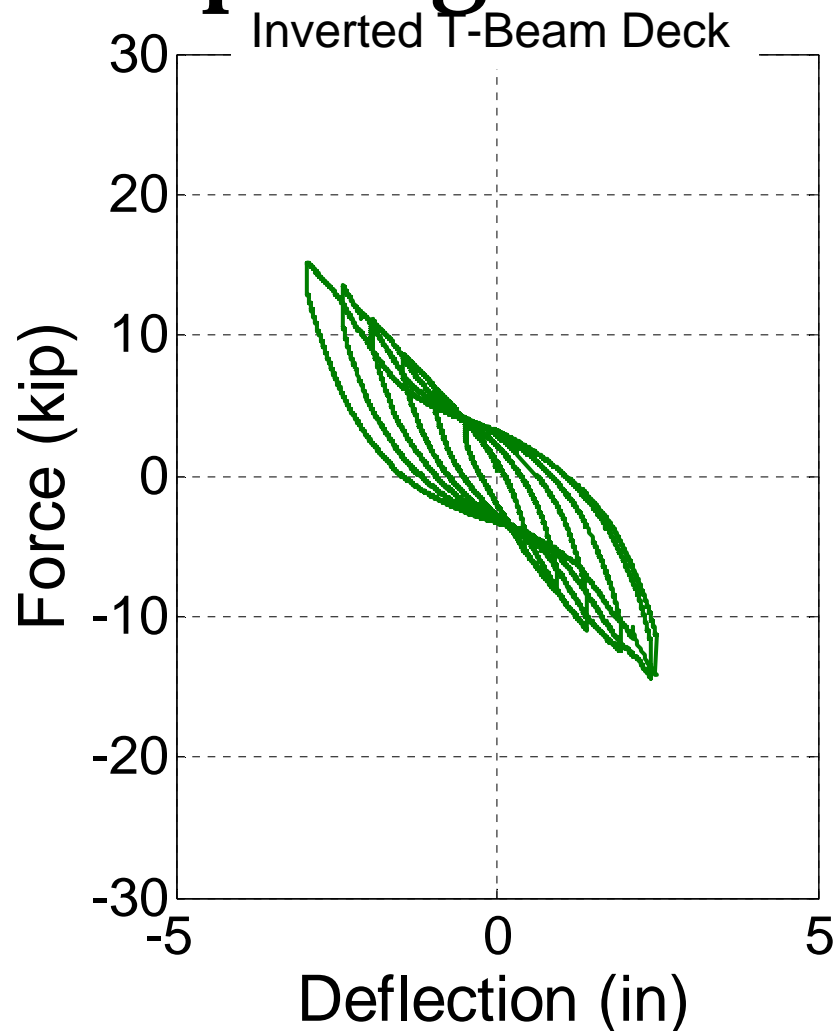
Deck to Girder Failure Modes



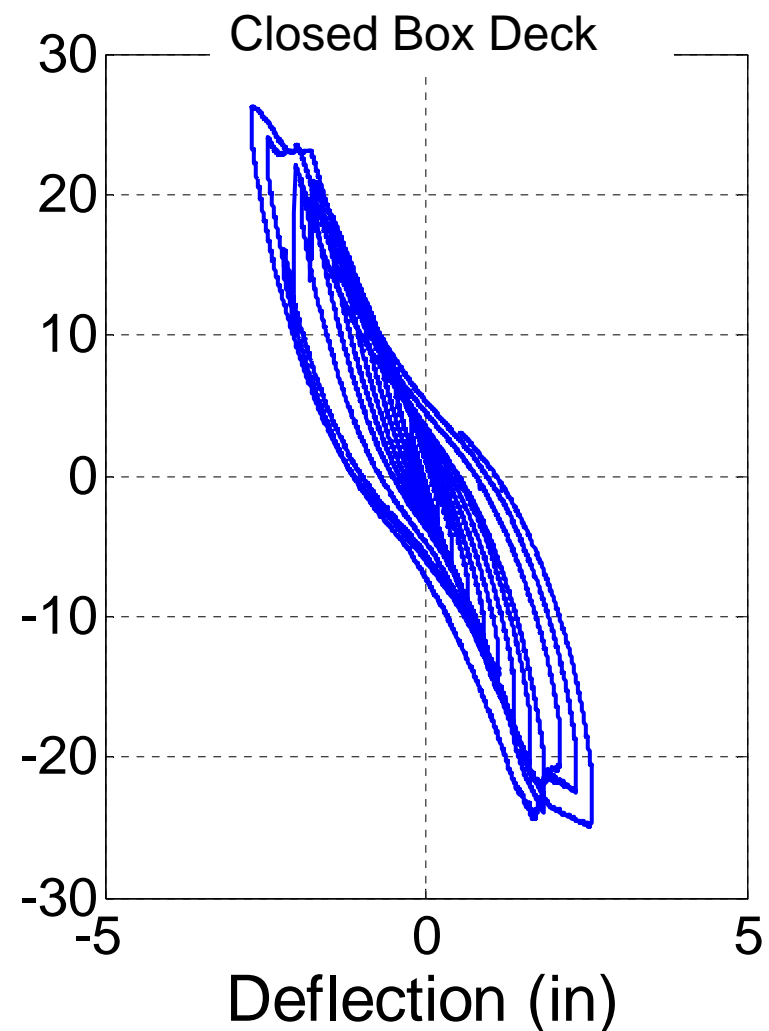
Diaphragm Test Setup



Diaphragm Test Results



Completed cycles to ± 2.5 in
Stiffness of approx. 6kip/in



Completed cycles to ± 2.5 in
Stiffness of approx. 12.6kip/in

Summary of Structural Evaluation

Monotonic Strength

- ultimate strength well above wheel load demands
- flexure and shear load failure mode via web/flange shear flow
- load sharing between panel beams distributes after initial failure
- minimal post failure residual displacement (maintenance?)

Fatigue

- HITEC evaluation approach unrealistic for high volume bridges such as the Morrison Bridge, use FHWA
- failure mode of monotonic to fatigue can change
- approximately 10% reduction in stiffness over life of Morrison Bridge deck

Bolted Connections

- FRP strength (local) controlled, not bolt
- direction dependent

Deck diaphragm stiffness doubles with closed deck





